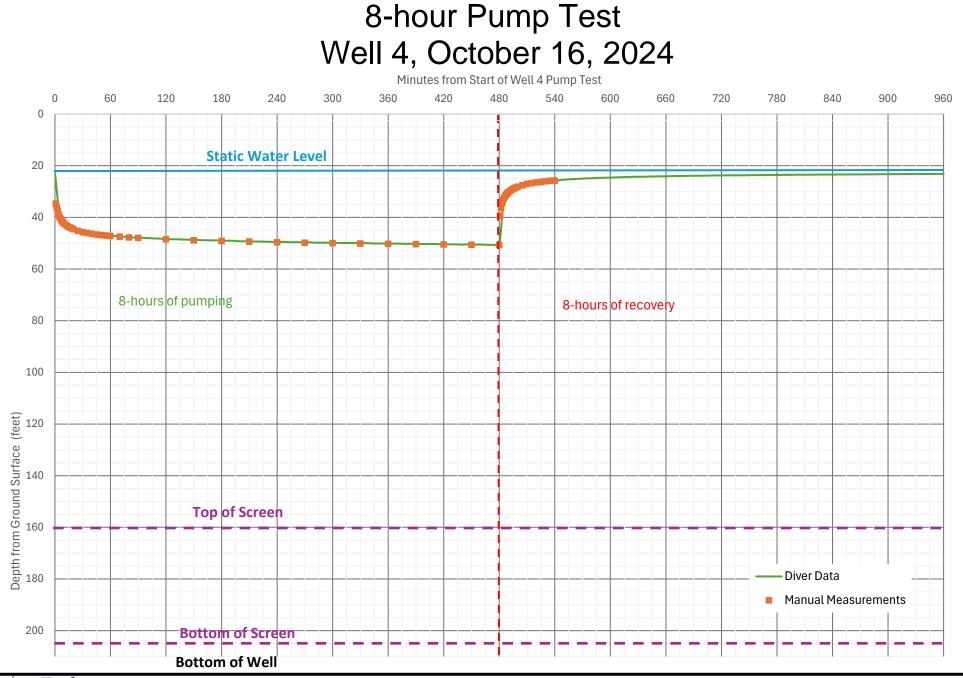




90-minute Preliminary Pump Test October 11, 2024

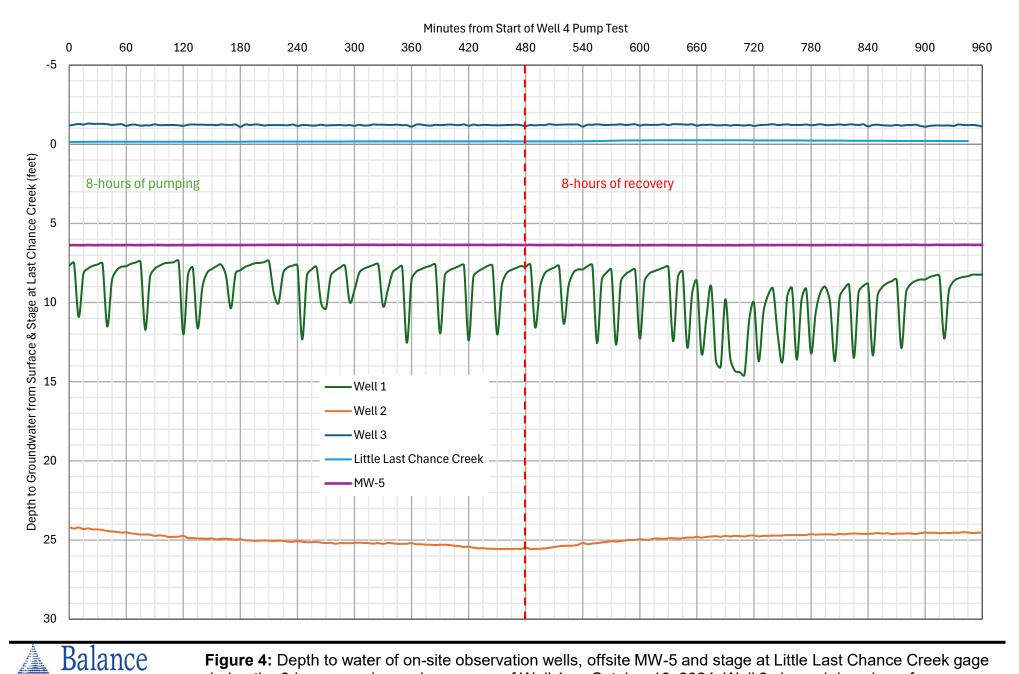
MEP Well ID	Well Depth (ft, bgs)	Screen Interval (feet)	Static Water Level (ft, bgs))	Pumping Rate (gpm)	90-minute Drawdown (ft)	Specific Capacity (gpm/ft)
Well 1	200	unknown	6.80	16.7	10.8	1.6
Well 2	205	unknown	24.80	60.0	22.4	2.7
Well 3	203	152-172	+1.00	61.0	33.85	1.8
Well 4	210	160-205	23.75	60.0	25.35	2.4

Table 1: Well construction information and 90-minute pump test data summary. (ft=feet; bgs= below ground surface;gpm=gallons per minute; gpd=gallons per day)



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Figure 3: 8-hour constant-rate pumping and recovery test of Well 4 on October 16, 2024, Meadow Edge Park, Plumas County, California. Well 4 was not pumped for 24 hours prior to the start of the 8-hour pump test at 54.7 gpm. 8-hour recovery period is shown after 480 minutes.



Balance Hydrologics Figure 4: Depth to water of on-site observation wells, offsite MW-5 and stage at Little Last Chance Creek gage during the 8-hour pumping and revcovery of Well 4 on October 16, 2024. Well 2 showed drawdown from pumping Well 4. Well 1 was used to supply water to Meadow Edge Park. LLE and MW-5 were not affected.

Given specifications:	<u>1 year</u>	<u>5 years</u>	<u>10 years</u>	20 years		
Hydraulic Conductivity, K=T/b (cm/s)	1.4E+02	1.4E+02	1.4E+02	1.4E+02		
Aquifer thickness, b (ft)	25	25	25	25		
Transmissivity, T (gpd/ft)	3500	3500	3500	3500		
Storativity, S	0.00025	0.00025	0.00025	0.00025		
Pumping rate, Q (gpm)	16.3	16.3	16.3	16.3		
Pumping duration (years)	1	5	10	20		
Pumping duration, t (days)	365.25	1826.25	3652.50	7305.00		
Calculate drawdown, s(r,t):						
Distance from well, r (ft)	<u>Drawdown, s (ft)</u>	<u>Drawdown, s (ft)</u>	<u>Drawdown, s (ft)</u>	<u>Drawdown, s (ft)</u>		
0.1	13.8	14.6	15.0	15.4		
1	11.3	12.2	12.5	12.9		
4	9.8	10.7	11.0	11.4		
10	8.8	9.7	10.1	10.4		
40	7.4	8.2	8.6	9.0		
100	6.4	7.2	7.6	8.0		
400	4.9	5.8	6.1	6.5		
1000	3.9	4.8	5.1	5.5		
2640	2.9	3.7	4.1	4.5		
5280	2.1	3.0	3.4	3.7		
	Distance	from well (ft)				
		100	1,000	10,000		
2 4 6 (1) 10 12 14 16 18 20 22			Existing demand: 1 years a Existing demand: 5 years a Existing demand: 10 years Existing demand: 20 years	at 16.3 gpm at 16.3 gpm		

Table 4. Theoretical 'cone-of-depression' drawdown calculations under existing conditions with no recharge, Meadow Edge Park, Plumas County, California

Method:

Theoretical drawdown was calculated using Cooper and Jacob modified nonequilibrium Theis equation

(Driscoll, F.G., 1986, Groundwater and Wells, 2nd Ed., p. 219).

Theis' nonequilibrium equation is based on the following assumptions:

a) The water-bearing formation is uniform in character and the hydraulic conductivity is the same in all directions.

b) The formation is uniform in thickness and infinite in areal extent.

c) The formation receives no recharge from any source.

d) The pumped well penetrates, and receives water from, the full thickness of the water-bearing formation.

e) The water removed from storage is discharged instantaneously when the head is lowered.

f) The pumping well is 100 percent efficient.

g) All water removed from the well comes from aquifer storage.

f) Laminar flow exists throughout the well and aquifer.

i) The water table or potentiometric surface has no slope.

Notes:

1. In a confined aquifer, the storativity (S) ranges in value from 0.005 to 0.00005. Storage for unconfined aquifers is known as specific yield (Sy) and generally varies between 0.01 and 0.30.

2. The modified nonequilibrium equation is valid for values of u less than about 0.05, otherwise values are approximate.

with no recharge, Meadow Edge Park, Plumas County, California						
Given specifications:	<u>1 year</u>	<u>5 years</u>	<u>10 years</u>	20 years		
Hydraulic Conductivity, K=T/b (cm/s)	1.4E+02	1.4E+02	1.4E+02	1.4E+02		
Aquifer thickness, b (ft)	25	25	25	25		
Transmissivity, T (gpd/ft)	3500	3500	3500	3500		
Storativity, S	0.00025	0.00025	0.00025	0.00025		
Pumping rate, Q (gpm)	26.3	26.3	26.3	26.3		
Pumping duration (years)	1	5	10	20		
Pumping duration, t (days)	365.25	1826.25	3652.50	7305.00		
Calculate drawdown, s(r,t):						
Distance from well, r (ft)	<u>Drawdown, s (ft)</u>	<u>Drawdown, s (ft)</u>	<u>Drawdown, s (ft)</u>	<u>Drawdown, s (ft)</u>		
0.1	22.2	23.6	24.2	24.8		
1	18.2	19.6	20.2	20.8		
4	15.8	17.2	17.8	18.4		
10	14.3	15.6	16.2	16.8		
40	11.9	13.3	13.9	14.4		
100	10.3	11.7	12.3	12.9		
400	7.9	9.3	9.9	10.5		
1000	6.3	7.7	8.3	8.9		
2640	4.6	6.0	6.6	7.2		
5280	3.5	4.8	5.4	6.0		
	Distance	from well (ft)				
1 10		100	1,000	10,000		
2						
4			A	A		
6						
(±) 0 u 10 pm 12 14 14		2				
				+ 26 2 mm		
16		Future proposed demand: 1 years at 26.3 gpm				
18		Future proposed demand: 5 years at 26.3 gpm				
20		Future proposed demand: 10 years at 26.3 gpm Future proposed demand: 20 years at 26.3 gpm				
22		Future pro	oposed demand: 20 years	at 26.3 gpm		

Table 5. Theoretical 'cone-of-depression' drawdown calculations under proposed conditions with no recharge, Meadow Edge Park, Plumas County, California

Method:

Theoretical drawdown was calculated using Cooper and Jacob modified nonequilibrium Theis equation

(Driscoll, F.G., 1986, Groundwater and Wells, 2nd Ed., p. 219).

Theis' nonequilibrium equation is based on the following assumptions:

a) The water-bearing formation is uniform in character and the hydraulic conductivity is the same in all directions.

b) The formation is uniform in thickness and infinite in areal extent.

c) The formation receives no recharge from any source.

d) The pumped well penetrates, and receives water from, the full thickness of the water-bearing formation.

e) The water removed from storage is discharged instantaneously when the head is lowered.

f) The pumping well is 100 percent efficient.

g) All water removed from the well comes from aquifer storage.

f) Laminar flow exists throughout the well and aquifer.

i) The water table or potentiometric surface has no slope.

Notes:

1. In a confined aquifer, the storativity (S) ranges in value from 0.005 to 0.00005. Storage for unconfined aquifers is known as specific yield (Sy) and generally varies between 0.01 and 0.30.

2. The modified nonequilibrium equation is valid for values of u less than about 0.05, otherwise values are approximate.

Table 2.Anticipated Water Demands

Demand	Existing Conditions	Proposed Conditions
Average Day Demand (ADD), with irrigation ²	23,420 gpd	37,920 gpd
Treruge Duy Demand (TDD), with Highlion	16.3 gpm	26.3 gpm
Max Day Demand (MDD), with irrigation	40,476 gpd	73,101 gpd
Max Day Demand (MDD), whit migation	28.1 gpm	50.8 gpm
Peak Hour Demand (PHD)	1,919 gph	3,959 gph
Teak Hour Demand (THD)	32.1 gpm	66.0 gpm

Demand estimates provided by Cranmer Engineering (2024), see Appendix A;

gpd = gallons per day; gph = gallons per hour; gpm = gallons per minute